

# **STORMWATER MANAGEMENT PLAN & DRAINAGE ANALYSIS**

**34 Clinton Avenue  
Village of Dobbs Ferry – New York  
Scheme C**

**January 19, 2023  
Revised March 22, 2023**



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**STORMWATER MANAGEMENT  
PLAN&DRAINAGE ANALYSIS  
34 Clinton Avenue  
Village of Dobbs Ferry – New York**

***INTRODUCTION***

This Stormwater Management Plan presents the proposed Best Management Practices (BMPs) to control erosion and sedimentation and manage stormwater during and upon construction of proposed multi-family residence, driveway, porch, patio, and walkway at 34 Clinton Avenue in the Village of Dobbs Ferry, Westchester County, New York.

This plan consists of this narrative and a plan set entitled: “Proposed Multi-Family Residence, 34 Clinton Avenue, Village of Dobbs Ferry, Westchester County - New York”, all as prepared by Hudson Engineering and Consulting, P.C., Elmsford, New York, dated March 22, 2023. The design is in accordance with the Village of Dobbs Ferry requirements. Since the project disturbance is less than one acre the New York State Department of Environmental Conservation [NYSDEC] stormwater regulations are not applicable.

***METHODOLOGY***

The stormwater analysis was developed utilizing the Soil Conservation Service (SCS) TR-20, 24-hour Type III storm events (HydroCad®) to assist with the design of the mitigating practices. The “Curve Number” (CN) value determination is based on soil type, vegetation and land use. The design is in accordance with the Village of Dobbs Ferry stormwater regulations. The “Time of Concentration” ( $T_c$ ) was determined as a direct entry of one-minute. The CN and  $T_c$  data are input into the computer model. The project site was modeled for the 100-year Type III – 24-hour storm event.

***PRE-DESIGN INVESTIGATIVE ANALYSIS***

A pre-design investigative analysis was performed including percolation and deep tests in the location shown on the plans.

Percolation tests were completed as follows: A 36-inch deep hole was excavated and an 8-inch percolation test hole was excavated approximately 24 inches in the hole. A 4-inch diameter pipe was inserted into the percolation hole and backfilled around. The hole was pre-soaked for 24 hours prior to running the tests. The pipe was filled with 24 inches of water and monitored for 1 hour or until the test-hole completely drained, whichever came first. The runs were repeated for a minimum of 4 runs and a consistent percolation rate. Percolation tests were performed in the vicinity of the potential stormwater mitigation practice [TP-1 and TP-2] until a constant rate was achieved, the result as follows:

- TP-1: A percolation rate of 0.79-minutes per inch (75.95-inches per hour) was observed. A rate of 20-inches per hour was utilized in the design.
- TP-2: A percolation rate of 1.88-minutes per inch (31.91-inches per hour) was observed. A rate of 20-inches per hour was utilized in the design.

Two (2) deep-hole test were also excavated and labeled [TP-1 and TP-2] as shown on the plan.

- TP-1 was excavated to a depth of 96-inches. The test revealed topsoil to a depth of 12-inches, clay from a depth of 30 to 36-inches, and sandy loam to the invert. Neither groundwater nor ledge rock was encountered.
- TP-2 was excavated to a depth of 96-inches. The test revealed topsoil to a depth of 12-inches, clay from a depth of 30 to 36-inches, and sandy loam to the invert. Neither groundwater nor ledge rock was encountered.

*The deep-hole test log and percolation test data sheets are attached.*

### **PRE-DEVELOPED CONDITION**

In the pre-developed condition, the site is characterized as sloping from the southeast to northwest. The soil classification, based upon Westchester County Soils Mapping is Paxton fine sandy loam, 3 to 8 percent slopes and Urban land-Charlton complex, 8 to 15 percent slopes. The site vegetation can be characterized as lawn and landscaped. The site is located along the west side of Clinton Avenue.

### **POST-DEVELOPED CONDITION**

Watershed 1 contains a tributary area of approximately 2,728 square feet, all of which are impervious in the form of proposed porch, a portion of the walkway, the front portion of the dwelling and walls. The CN value for this area is 98 and the Time of Concentration (Tc) is a direct entry of 1 minute. The runoff from this tributary area is conveyed via a comprehensive drainage system to six (6) Cultec Recharger 330XLHD units set in 12 inches at all sides and 6 inches at the invert. The system is designed to fully accept (no release) the entire stormwater runoff volume for the 100-year storm event from the watershed and ex-filtrate the runoff into the surrounding soil sub-strata.

Watershed 2 contains a tributary area of approximately 7,319 square feet, all of which are impervious in the form of proposed permeable driveway, a portion of the walkway, a portion of the dwelling, walls, rear patio, and future parking (311 S.F.). The CN value for this area is 98 and the Time of Concentration (Tc) is a direct entry of 1 minute. The runoff from this tributary area is conveyed via a comprehensive drainage system to sixteen (16) Cultec Recharger 330XLHD units set in 12 inches at all sides and 6 inches at the invert. The system is designed to fully accept (no release) the entire stormwater runoff volume for the

100-year storm event from the watershed and ex-filtrate the runoff into the surrounding soil sub-strata.

## ***EROSION AND SEDIMENT CONTROL COMPONENTS***

The primary aim of the soil and sediment control measures is to reduce soil erosion from areas stripped of vegetation during and after construction and to prevent silt from reaching the off-site drainage structures and downstream properties. The Sediment and Erosion Control Components are an integral component of the construction sequencing and will be implemented to control sedimentation and re-establish vegetation.

Planned erosion and sedimentation control practices during construction include the installation, inspection and maintenance of the inlet protection, soil stockpile areas, and diversion swales and silt fencing. General land grading practices, including land stabilization and construction sequencing are also integrated into the Sediment and Erosion Control Plan. Dust control is not expected to be a problem due to the relatively limited area of exposure, the undisturbed perimeter of trees around the project area and the relatively short time of exposure. Should excessive dust be generated, it will be controlled by sprinkling.

All proposed soil erosion and sediment control practices have been designed in accordance with the following publications:

- New York State standards and Specifications for Erosion and Sediment Control, November 2016
- New York State General Permit for Stormwater Discharges, GP-0-20-001 (General permit).
- “Reducing the Impacts of Stormwater Runoff from New Development”, as published by the New York State Department of Environmental Conservation (NYSDEC), second edition, April, 1993.

The proposed soil erosion and sediment control devices include the planned erosion control practices outlined below. Maintenance procedures for each erosion control practice have also been outlined below.

### **• SILT FENCE**

Silt fence (geo-textile filter cloth) shall be placed in locations depicted on the approved plans. The purpose of the silt fence is to reduce the velocity of sediment laden stormwater from small drainage areas and to intercept the transported sediment load. In general, silt fence shall be used at the toe of slopes or intermediately within slopes where obvious channel concentration of stormwater is not present.

## Maintenance

Silt fencing shall be inspected at a minimum of once per week and prior to and within 24 hours following a rain event  $\frac{1}{2}$ " or greater. Inspections shall include ensuring that the fence material is tightly secured to the woven wire and the wire is secured to the wood posts. In addition, overlapping filter fabric shall be secured and the fabric shall be maintained a minimum of six (6) inches below grade. In the event that any "bulges" develop in the fence, that section of fence shall be replaced within 24 hours with new fence section. Any sediment build-up against the fence shall be removed within 24 hours and deposited on-site a minimum of 100 feet outside of any wetland or watercourse.

The installation of silt fencing will be maintained or replaced until the fencing is no longer necessary. Once the site is stabilized, all silt fences shall be removed. The immediate area occupied by the silt fence will be shaped to an acceptable grade and stabilized.

### • **INLET PROTECTION**

After catch basins and surface inlets have been installed, these drain inlets will receive stormwater from the roadways, driveways, and surrounding overland watersheds. In order to protect the receiving waters from sedimentation, the contractor shall install stone and block inlet protection as shown on the plans. Once installed,  $\frac{3}{4}$  inch stone aggregate shall be installed around the perimeter of all catch basins and surface inlets as illustrated on the approved plans. This barrier will allow stormwater to be filtered prior to reaching the basin inlet grate.

The stone barrier should have a minimum height of 1 foot and a maximum height of 2 feet. Do not use mortar. The height should be limited to prevent excess ponding and bypass flow. Recess the first course of blocks at least 2 inches below the crest opening of the storm drain for lateral support. Subsequent courses can be supported laterally if needed by placing a 2x4 inch wood stud through the block openings perpendicular to the course. The bottom row should have a few blocks oriented so flow can drain through the block to dewater the basin area. The stone should be placed just below the top of the blocks on slopes of 2:1 or flatter. Place hardware cloth or wire mesh with  $\frac{1}{2}$  inch openings over all block openings to hold stone in place.

As an optional design, the concrete blocks may be omitted and the entire structure constructed of stone, ringing the outlet ("doughnut"). The stone should be kept at a 3:1 slope toward the inlet to keep it from being washed into the inlet.

A level area 1 foot wide and four inches below the crest will further prevent wash. Stone on the slope toward the inlet should be at least 3 inches in size

for stability and 1 inch or smaller away from the inlet to control flow rate. The elevation of the top of the stone crest must be maintained 6 inches lower than the ground elevation down slope from the inlet to ensure that all storm flows pass over the stone into the storm drain and not past the structure.

The barrier should be inspected after each rain event and repairs made within 24 hours. Remove sediment as necessary to provide for accurate storage volume for subsequent rains. Upon stabilization of contributing drainage area, remove all materials and any unstable soil and dispose of properly. Bring the disturbed area to proper grade, smooth, compact and stabilized in a manner appropriate to the site.

### Maintenance

Stone Aggregate: The stone aggregate shall be inspected weekly prior to and within 24 hours following a rain event ½" or greater. Care shall be taken to ensure that all stone aggregate is properly located and secure and do not become displaced. The stone aggregate shall be inspected for accumulated sediments and any accumulated sediment shall be removed from the device and deposited not less than 100 feet from wetland or watercourse.

- **TREE PROTECTION**

All significant trees to be preserved located within the limits of disturbance and on the perimeter of the disturbance limits shall be protected from harm by erecting a 3' high (minimum) snow fence completely surrounding the tree. Snow fence should extend to the drip-line of the tree to be preserved. Trees designated to be protected shall be identified during the staking of the limits of disturbance for each construction phase.

### Maintenance

The snow fence shall be inspected daily to ensure that the perimeter of the fence remains at the drip-line of the tree to be preserved. Any damaged portions of the fence shall be repaired or replaced within 24 hours. Care shall also be taken to ensure that no construction equipment is driven or parked within the drip-line of the tree to be preserved.

- **SOIL/SHOT ROCK STOCKPILING**

All soil and shot rock stripped from the construction area during grubbing and mass grading shall be stockpiled in locations shown on the plans, but in no case shall they be placed within 100' of a wetland or watercourse. The stockpiled soils shall be re-used during finish-grading to provide a suitable growing medium for plant establishment. Soil stockpiles shall be protected from erosion by vegetating the stockpile with rapidly –germinating grass seed

(during the May 1<sup>st</sup> – October 30<sup>th</sup>) planting season or covering the stockpile with tarpaulin the remainder of the year. Install silt fence around toe of slope.

### Maintenance

Sediment controls (silt fence) surrounding the stockpiles shall be inspected according to the recommended maintenance outline above. *All stockpiles shall be inspected for signs of erosion or problems with seed establishment weekly or tarpaulin and prior to and within 24 hours following a rain event ½" or greater.*

- **GENERAL LAND GRADING**

The intent of the Erosion & Sediment Control Plan is to control disturbed areas such that soils are protected from erosion by temporary methods and, ultimately, by permanent vegetation. Where practicable, all cut and fill slopes shall be kept to a maximum slope of 2:1. In the event that a slope must exceed a 2:1 slope, it will be stabilized with stone riprap. On fill slopes, all material will be placed in layers not to exceed 12 inches in depth and adequately compacted. Diversion swales shall be constructed on the top of all fill embankments to divert any overland flows away from the fill slopes.

- **SURFACE STABILIZATION**

All disturbed areas will be protected from erosion with the use of vegetative measures (i.e., grass seed mix, sod) hydro mulch netting or hay. When activities temporarily cease during construction, soil stockpiles and exposed soil should be stabilized by seed, mulch or other appropriate measures within 7 days after construction activity has ceased, or 24 hours prior to a rain event ½" or greater.

All seeded areas will be re-seeded areas as necessary and mulched according to the site plan to maintain a vigorous, dense vegetative cover,

Erosion control barriers (silt fencing) shall be placed around exposed areas during construction. Where exposed areas are immediately uphill from a wetland or watercourse, the erosion control barrier will consist of double rows of silt fencing. Any areas stripped of vegetation during construction will be vegetated and/or mulch, but in no case more than 14 days to prevent erosion of the exposed soils. And topsoil removed during construction will be temporarily stockpiled for future use in grading and landscaping.

As mentioned above, temporary vegetation will be established to protect exposed soil areas during construction. If growing conditions are not suitable for the temporary vegetation, mulch will be used to the satisfaction of the Town Engineer. Materials that may be used for mulching include straw, hay, salt hay, wood fiber, synthetic soil stabilizers, mulch netting, sod or hydro mulch. In site areas where significant erosion potential exists (steep slopes)



and where specifically directed by the Town's representative, Curlex Excelsior erosion control blankets (manufactured by American Excelsior, or approved equal) shall be installed. A permanent vegetative cover will be established upon completion of construction of those areas that have been brought to finish-grade and to remain undisturbed.

- **Temporary Stabilization(May 1<sup>st</sup> through October 31st planting season)**

The following seeding application should be used depending on the time of year.

- Spring/summer or early fall, seed the area with ryegrass (annual or perennial) at 30 lbs. per acre (Approximately 0.7 lb/1000 sq. ft. or use 1 lb/1000 sq. ft.).
- Late fall or early winter, seed Certified 'Aroostook' winter rye (cereal rye) at 100 lbs. per acre (2.5 lbs/1000 sq. ft.).

- **Permanent Stabilization(May 1<sup>st</sup> through October 31st planting season)**

1. Provide minimum of four (4) inches topsoil for all new lawn areas. Top dress all existing disturbed lawn areas with two (2) inches of topsoil.
2. Grass seed shall be evenly sown by mechanical seeder at a rate of 3.0-4.0 pounds per 1,000 square feet.
3. Fine rake, roll and water to a depth of one inch all seeded areas.
4. Apply air-dried hay or straw mulch to provide 90% coverage of surface (approximately 90 lbs. per 1,000 SF). Use small grain straw where mulch is maintained for more than three months
5. Contractor shall provide, at his own expense, protection against trespassing and other damage to lawn areas.
6. Lawn seed mix shall include:
  - a. General Recreation areas and lawns:
    - 65% Kentucky Bluegrass blend
    - 20% Perennial Rye
    - 15% Fine fescue

Sod may be used as an alternate to seeding in select areas.

Slow release fertilizers will be applied by hand to horticultural plantings as part of regular horticultural maintenance program and shall be limited to a single spring application.



## **CONSTRUCTION PRACTICES TO MINIMIZE STORMWATER CONTAMINATION**

Adequate measures shall be taken to minimize contaminant particles arising from the discharge of solid materials, including building materials, grading operations, and the reclamation and placement of pavement, during project construction, including but not limited to:

- Building materials, garbage, and debris shall be cleaned up daily and deposited into dumpsters, which will be periodically removed from the site and appropriately disposed of.
- Dump trucks hauling material from the construction site will be covered with a tarpaulin.
- The paved street adjacent to the site entrance will be swept daily to remove excess mud, dirt, or rock tracked from the site.
- Petroleum products will be stored in tightly sealed containers that are clearly labeled.
- All vehicles on site will be monitored for leaks and receive regular preventive maintenance to reduce the chance of leakage.
- All spills will be cleaned up immediately upon discovery. Spills large enough to reach the storm system will be reported to the NationalResponseCenter at 1-800-424-8802.
- Materials and equipment necessary for spill cleanup will be kept in the temporary material storage trailer onsite. Equipment will include, but not be limited to, brooms, dust pans, mops, rags, gloves, goggles, kitty litter, sand, saw dust, and plastic and metal trash containers.
- All paint containers and curing compounds will be tightly sealed and stored when not required for use. Excess paint will not be discharged to the storm system, but will be properly disposed according to the manufacturer's instructions.
- Sanitary waste will be collected from portable units a minimum of two times a week to avoid overfilling.
- Any asphalt substances used on-site will be applied according to the manufacturer's recommendation.
- Fertilizers will be stored in a covered shed and partially used bags will be transferred to a sealable bin to avoid spills and will be applied only in the minimum amounts recommended by the manufacturer and worked into the soil to limit exposure to stormwater.

- No disturbed area shall be left un-stabilized for longer than 14 days during the growing season.
- When erosion is likely to be a problem, grubbing operations shall be scheduled and performed such that grading operations and permanent erosion control features can follow within 24 hours thereafter.
- As work progresses, patch seeding shall be done as required on areas previously treated to maintain or establish protective cover.
- Drainage pipes and swales/ditches shall generally be constructed in a sequence from outlet to inlet in order to stabilize outlet areas and ditches before water is directed to the new installation or any portion thereof, unless conditions unique to the location warrant an alternative method.

### ***STORMWATER MANAGEMENT FACILITIES MAINTENANCE PROGRAM***

The following maintenance plan has been developed to maintain the proper function of all drainage and erosion and sediment control facilities:

- Minimize the use of road salt for maintenance of driveway areas.
- Drainage inlets shall be vacuum swept twice a year, at the conclusion of the landscape season in the fall and at the conclusion of the sand and de-icing season in the spring. Inspect exfiltration/attenuation gallery for sediment and remove same if found.

The permanent maintenance program will be managed by the future homeowners upon completion of construction and acceptance of the improvements.

### ***CONCLUSION***

The stormwater management plan proposed meets all the requirements set forth by the Village of Dobbs ferry. Design modification requirements that may occur during the approval process will be performed and submitted for review to the Village of Dobbs Ferry.

# Extreme Precipitation Tables

## Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing	Yes
State	New York
Location	
Longitude	73.872 degrees West
Latitude	41.013 degrees North
Elevation	0 feet
Date/Time	Tue, 17 Jan 2023 14:37:20 -0500

## Extreme Precipitation Estimates

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.33	0.51	0.63	0.82	1.03	1.29	1yr	0.89	1.23	1.48	1.84	2.27	2.81	3.20	1yr	2.49	3.08	3.57	4.29	4.93	1yr
2yr	0.40	0.62	0.77	1.01	1.27	1.59	2yr	1.10	1.49	1.83	2.26	2.78	3.42	3.85	2yr	3.03	3.70	4.26	5.05	5.73	2yr
5yr	0.47	0.73	0.91	1.23	1.57	1.98	5yr	1.35	1.84	2.29	2.84	3.49	4.28	4.87	5yr	3.78	4.68	5.43	6.33	7.08	5yr
10yr	0.53	0.83	1.04	1.42	1.84	2.34	10yr	1.59	2.17	2.72	3.37	4.14	5.06	5.82	10yr	4.48	5.59	6.53	7.50	8.32	10yr
25yr	0.61	0.97	1.24	1.71	2.28	2.93	25yr	1.97	2.69	3.41	4.24	5.22	6.34	7.37	25yr	5.61	7.09	8.33	9.40	10.29	25yr
50yr	0.69	1.11	1.43	2.00	2.68	3.48	50yr	2.32	3.17	4.06	5.05	6.19	7.51	8.82	50yr	6.65	8.48	10.03	11.15	12.10	50yr
100yr	0.78	1.27	1.64	2.32	3.16	4.13	100yr	2.73	3.74	4.82	6.01	7.36	8.91	10.55	100yr	7.89	10.15	12.07	13.23	14.23	100yr
200yr	0.89	1.45	1.88	2.70	3.73	4.90	200yr	3.22	4.41	5.73	7.15	8.76	10.58	12.63	200yr	9.36	12.15	14.54	15.70	16.74	200yr
500yr	1.07	1.75	2.29	3.32	4.65	6.14	500yr	4.01	5.50	7.21	9.00	11.02	13.28	16.04	500yr	11.75	15.42	18.61	19.70	20.76	500yr

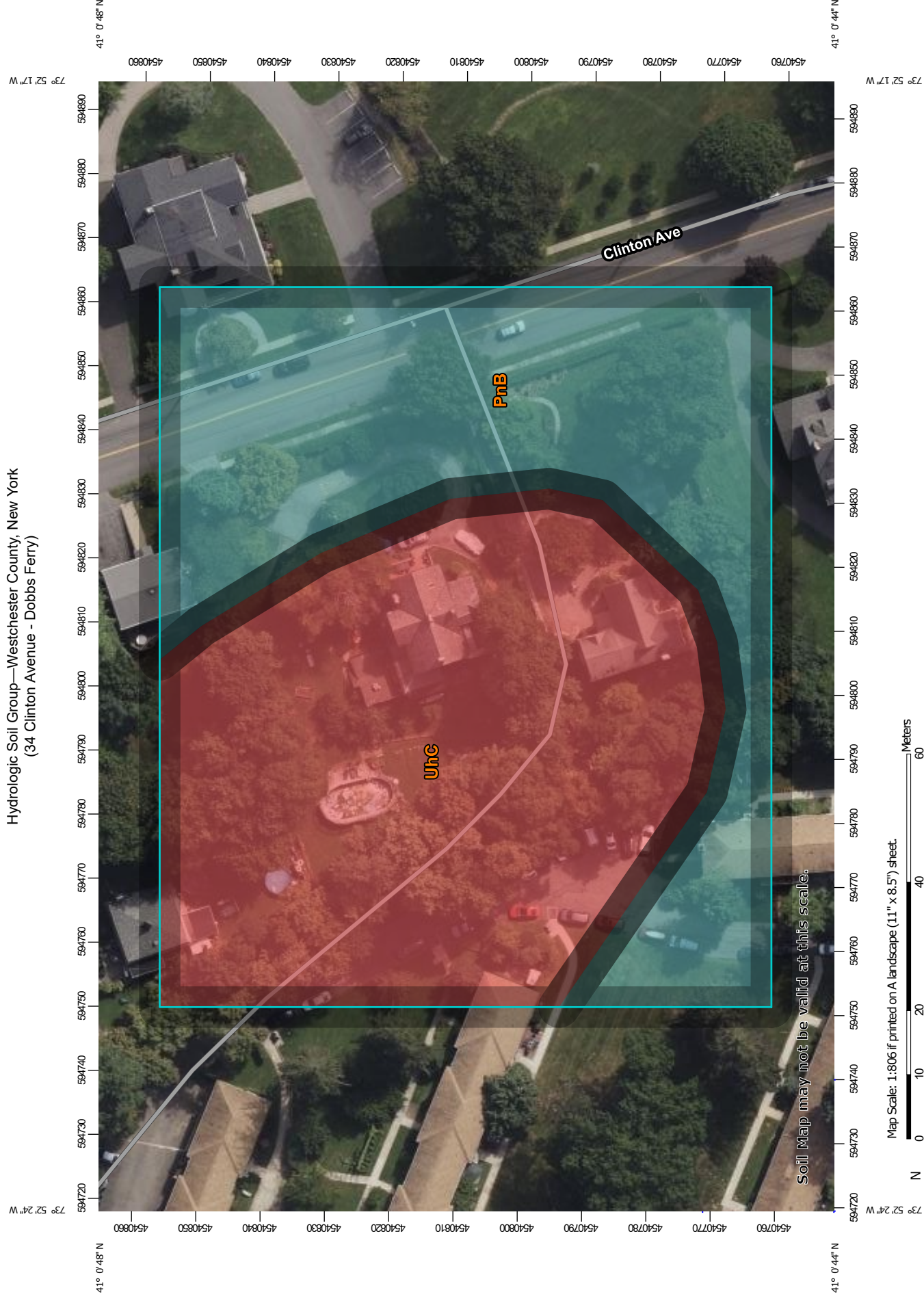
## Lower Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.30	0.46	0.56	0.75	0.92	1.13	1yr	0.80	1.11	1.31	1.55	2.17	2.44	2.62	1yr	2.16	2.52	3.29	3.99	4.37	1yr
2yr	0.38	0.59	0.73	0.98	1.21	1.46	2yr	1.05	1.43	1.67	2.17	2.69	3.32	3.73	2yr	2.94	3.59	4.12	4.88	5.55	2yr
5yr	0.41	0.64	0.79	1.08	1.38	1.70	5yr	1.19	1.67	1.96	2.51	3.16	3.97	4.51	5yr	3.51	4.34	4.99	5.82	6.52	5yr
10yr	0.44	0.68	0.84	1.18	1.52	1.89	10yr	1.31	1.85	2.19	2.75	3.56	4.55	5.20	10yr	4.03	5.00	5.73	6.62	7.28	10yr
25yr	0.48	0.73	0.90	1.29	1.69	2.17	25yr	1.46	2.12	2.54	3.09	4.17	5.47	6.26	25yr	4.85	6.02	6.85	7.89	8.32	25yr
50yr	0.50	0.76	0.95	1.36	1.83	2.40	50yr	1.58	2.34	2.84	3.35	4.69	6.30	7.21	50yr	5.58	6.93	7.79	8.98	9.12	50yr
100yr	0.52	0.79	0.99	1.43	1.96	2.63	100yr	1.69	2.57	3.16	3.60	5.25	7.27	8.29	100yr	6.43	7.97	8.90	10.23	9.94	100yr
200yr	0.54	0.81	1.02	1.48	2.07	2.87	200yr	1.78	2.81	3.53	3.84	5.91	8.41	9.52	200yr	7.44	9.16	10.14	11.65	10.77	200yr
500yr	0.55	0.83	1.06	1.54	2.19	3.23	500yr	1.89	3.15	4.08	4.13	6.93	10.21	11.37	500yr	9.03	10.93	12.01	13.81	11.88	500yr

## Upper Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.37	0.58	0.70	0.95	1.16	1.39	1yr	1.01	1.36	1.61	2.11	2.49	3.03	3.49	1yr	2.68	3.36	3.86	4.61	5.22	1yr
2yr	0.42	0.65	0.79	1.08	1.33	1.60	2yr	1.14	1.56	1.82	2.36	2.90	3.55	4.00	2yr	3.14	3.84	4.46	5.23	6.03	2yr
5yr	0.52	0.81	1.00	1.37	1.75	2.04	5yr	1.51	2.00	2.36	3.06	3.81	4.60	5.26	5yr	4.07	5.06	5.85	6.83	7.63	5yr
10yr	0.63	0.97	1.20	1.68	2.17	2.49	10yr	1.87	2.43	2.88	3.74	4.67	5.60	6.52	10yr	4.96	6.27	7.24	8.37	9.28	10yr
25yr	0.82	1.24	1.55	2.21	2.91	3.24	25yr	2.51	3.17	3.76	4.95	6.16	7.24	8.64	25yr	6.41	8.31	9.61	10.99	12.01	25yr
50yr	1.00	1.52	1.89	2.72	3.66	3.99	50yr	3.15	3.90	4.60	6.14	7.59	8.81	10.70	50yr	7.79	10.29	11.92	13.49	14.62	50yr
100yr	1.23	1.86	2.33	3.36	4.61	4.90	100yr	3.98	4.79	5.66	7.64	9.37	10.71	13.28	100yr	9.48	12.77	14.84	16.57	17.84	100yr
200yr	1.52	2.28	2.89	4.19	5.84	6.03	200yr	5.04	5.89	6.96	9.55	11.56	13.02	16.47	200yr	11.53	15.84	18.48	20.37	21.79	200yr
500yr	2.04	3.03	3.90	5.67	8.06	7.96	500yr	6.96	7.78	9.15	12.89	15.28	16.84	21.94	500yr	14.91	21.10	24.74	26.75	28.43	500yr

# Hydrologic Soil Group—Westchester County, New York (34 Clinton Avenue - Dobbs Ferry)



MAP LEGEND

**Area of Interest (AOI)**

Area of Interest (AOI)

**Soils**

**Soil Rating Polygons**

A

A/D

B

B/D

C

C/D

D

Not rated or not available

**Soil Rating Lines**

A

A/D

B

B/D

C

C/D

D

Not rated or not available

**Soil Rating Points**

A

A/D

B

B/D

**Water Features**

Streams and Canals

**Transportation**

Rails

Interstate Highways

US Routes

Major Roads

Local Roads

**Background**

Aerial Photography

C

C/D

D

Not rated or not available

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

**Warning:** Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
Web Soil Survey URL:  
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Westchester County, New York  
Survey Area Data: Version 18, Sep 10, 2022

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Apr 13, 2021—Sep 14, 2021

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.



## Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
PnB	Paxton fine sandy loam, 3 to 8 percent slopes	C	1.2	45.7%
UhC	Urban land-Charlton complex, 8 to 15 percent slopes	D	1.4	54.3%
<b>Totals for Area of Interest</b>			<b>2.7</b>	<b>100.0%</b>

## Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

## Rating Options

### *Aggregation Method: Dominant Condition*

Aggregation is the process by which a set of component attribute values is reduced to a single value that represents the map unit as a whole.

A map unit is typically composed of one or more "components". A component is either some type of soil or some nonsoil entity, e.g., rock outcrop. For the attribute being aggregated, the first step of the aggregation process is to derive one attribute value for each of a map unit's components. From this set of component attributes, the next step of the aggregation process derives a single value that represents the map unit as a whole. Once a single value for each map unit is derived, a thematic map for soil map units can be rendered. Aggregation must be done because, on any soil map, map units are delineated but components are not.

For each of a map unit's components, a corresponding percent composition is recorded. A percent composition of 60 indicates that the corresponding component typically makes up approximately 60% of the map unit. Percent composition is a critical factor in some, but not all, aggregation methods.

The aggregation method "Dominant Condition" first groups like attribute values for the components in a map unit. For each group, percent composition is set to the sum of the percent composition of all components participating in that group. These groups now represent "conditions" rather than components. The attribute value associated with the group with the highest cumulative percent composition is returned. If more than one group shares the highest cumulative percent composition, the corresponding "tie-break" rule determines which value should be returned. The "tie-break" rule indicates whether the lower or higher group value should be returned in the case of a percent composition tie. The result returned by this aggregation method represents the dominant condition throughout the map unit only when no tie has occurred.

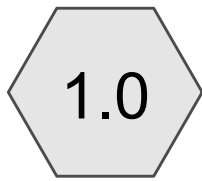
### *Component Percent Cutoff: None Specified*

Components whose percent composition is below the cutoff value will not be considered. If no cutoff value is specified, all components in the database will be considered. The data for some contrasting soils of minor extent may not be in the database, and therefore are not considered.

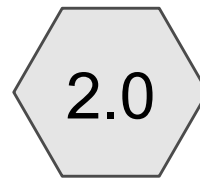
### *Tie-break Rule: Higher*

The tie-break rule indicates which value should be selected from a set of multiple candidate values, or which value should be selected in the event of a percent composition tie.

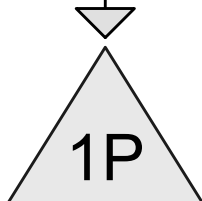




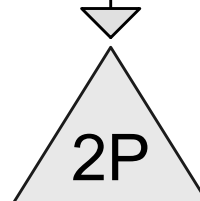
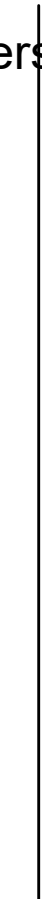
Watershed 1



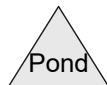
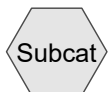
Watershed 2



6 Cultec R-330XLHD



16 Cultec R-330XLHD



**Routing Diagram for 34 Clinton Avenue**

Prepared by Hudson Engineering & Consulting, P.C., Printed 3/22/2023  
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## 34 Clinton Avenue

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### Rainfall Events Listing (selected events)

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	100-Year	Type III 24-hr		Default	24.00	1	8.91	2

### 34 Clinton Avenue

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#### Area Listing (all nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
451	98	Front Patio & Walkways (1.0)
84	98	Front Porch (1.0)
2,128	98	Front Portion of Dwelling (1.0)
500	98	Front Walkway (2.0)
77	98	Front Walls (2.0)
65	98	Front wall (1.0)
311	98	Future parking (2.0)
2,192	98	Permeable drive (2.0)
3,885	98	Portion of Dwelling (2.0)
354	98	Rear Patio (2.0)
<b>10,047</b>	<b>98</b>	<b>TOTAL AREA</b>

### 34 Clinton Avenue

Type III 24-hr 100-Year Rainfall=8.91"

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Time span=0.00-60.00 hrs, dt=0.01 hrs, 6001 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

#### Subcatchment 1.0: Watershed 1

Runoff Area=2,728 sf 100.00% Impervious Runoff Depth=8.67"  
Tc=1.0 min CN=98 Runoff=0.65 cfs 1,971 cf

#### Subcatchment 2.0: Watershed 2

Runoff Area=7,319 sf 100.00% Impervious Runoff Depth=8.67"  
Tc=1.0 min CN=98 Runoff=1.75 cfs 5,288 cf

#### Pond 1P: 6 Cultec R-330XLHD

Peak Elev=2.30' Storage=389 cf Inflow=0.65 cfs 1,971 cf  
Outflow=0.13 cfs 1,971 cf

#### Pond 2P: 16 Cultec R-330XLHD

Peak Elev=2.75' Storage=1,145 cf Inflow=1.75 cfs 5,288 cf  
Outflow=0.30 cfs 5,288 cf

**Total Runoff Area = 10,047 sf Runoff Volume = 7,259 cf Average Runoff Depth = 8.67"**  
**0.00% Pervious = 0 sf 100.00% Impervious = 10,047 sf**

## 34 Clinton Avenue

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Type III 24-hr 100-Year Rainfall=8.91"

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### Summary for Subcatchment 1.0: Watershed 1

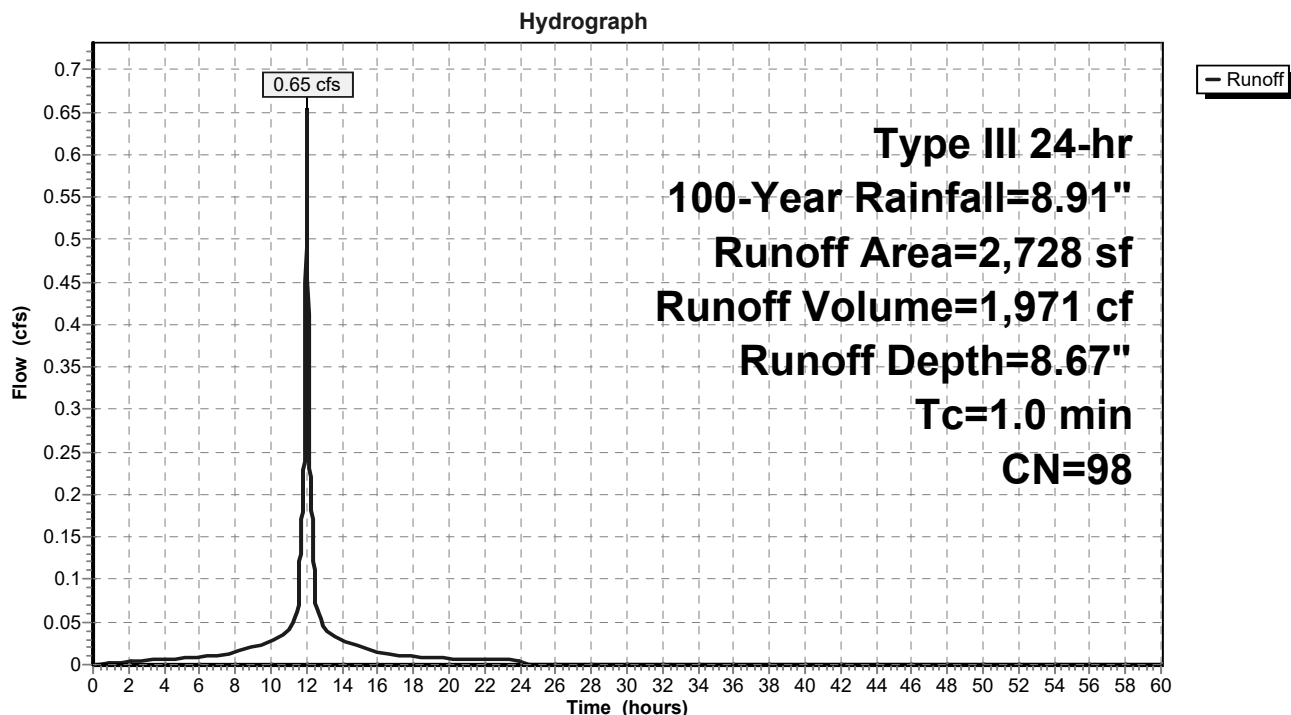
Runoff = 0.65 cfs @ 12.01 hrs, Volume= 1,971 cf, Depth= 8.67"  
Routed to Pond 1P : 6 Cultec R-330XLHD

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs  
Type III 24-hr 100-Year Rainfall=8.91"

	Area (sf)	CN	Description
*	2,128	98	Front Portion of Dwelling
*	84	98	Front Porch
*	451	98	Front Patio & Walkways
*	65	98	Front wall
	2,728	98	Weighted Average
	2,728		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.0					Direct Entry,

### Subcatchment 1.0: Watershed 1



## 34 Clinton Avenue

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Type III 24-hr 100-Year Rainfall=8.91"

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### Summary for Subcatchment 2.0: Watershed 2

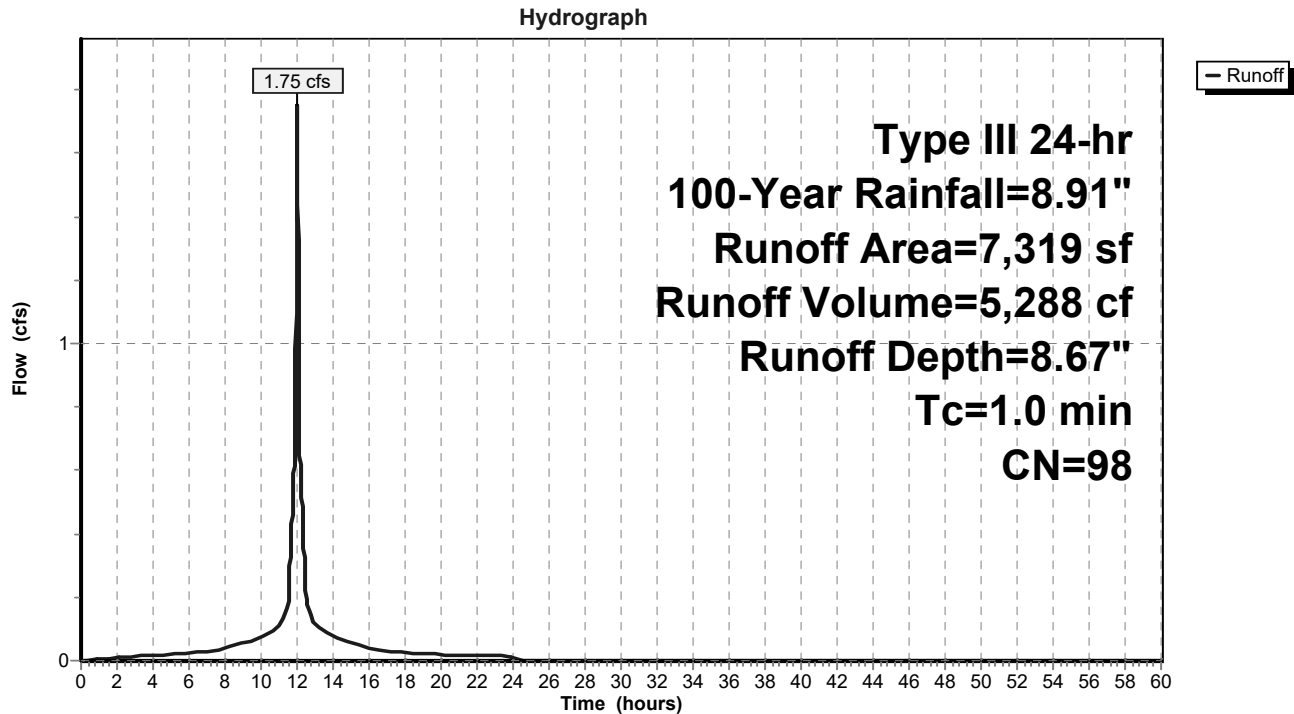
Runoff = 1.75 cfs @ 12.01 hrs, Volume= 5,288 cf, Depth= 8.67"  
Routed to Pond 2P : 16 Cultec R-330XLHD

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs  
Type III 24-hr 100-Year Rainfall=8.91"

	Area (sf)	CN	Description
*	3,885	98	Portion of Dwelling
*	354	98	Rear Patio
*	2,192	98	Permeable drive
*	500	98	Front Walkway
*	77	98	Front Walls
*	311	98	Future parking
	7,319	98	Weighted Average
	7,319		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.0					Direct Entry,

### Subcatchment 2.0: Watershed 2



**34 Clinton Avenue**

Type III 24-hr 100-Year Rainfall=8.91"

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**Summary for Pond 1P: 6 Cultec R-330XLHD**

Inflow Area = 2,728 sf, 100.00% Impervious, Inflow Depth = 8.67" for 100-Year event  
 Inflow = 0.65 cfs @ 12.01 hrs, Volume= 1,971 cf  
 Outflow = 0.13 cfs @ 11.65 hrs, Volume= 1,971 cf, Atten= 81%, Lag= 0.0 min  
 Discarded = 0.13 cfs @ 11.65 hrs, Volume= 1,971 cf

Routing by Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs  
 Peak Elev= 2.30' @ 12.39 hrs Surf.Area= 274 sf Storage= 389 cf

Plug-Flow detention time= 13.7 min calculated for 1,971 cf (100% of inflow)  
 Center-of-Mass det. time= 13.7 min ( 749.0 - 735.3 )

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	149 cf	<b>11.17'W x 24.50'L x 3.04'H Field A</b> 832 cf Overall - 335 cf Embedded = 497 cf x 30.0% Voids
#2A	0.50'	335 cf	<b>Cultec R-330XLHD x 6 Inside #1</b> Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
		484 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	<b>20.000 in/hr Exfiltration over Surface area</b>

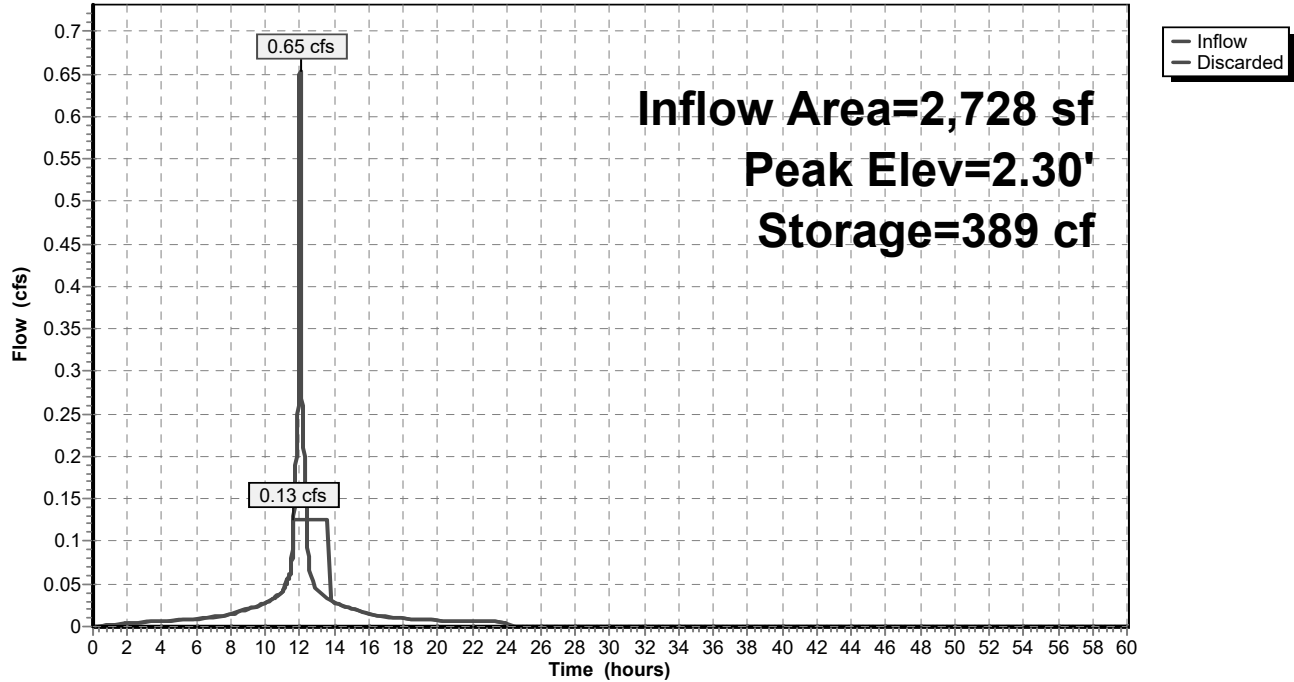
Discarded OutFlow Max=0.13 cfs @ 11.65 hrs HW=0.03' (Free Discharge)

↑1=Exfiltration (Exfiltration Controls 0.13 cfs)



**Pond 1P: 6 Cultec R-330XLHD**

Hydrograph



**34 Clinton Avenue**

Type III 24-hr 100-Year Rainfall=8.91"

Prepared by Hudson Engineering &amp; Consulting, P.C.

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**Summary for Pond 2P: 16 Cultec R-330XLHD**

Inflow Area = 7,319 sf, 100.00% Impervious, Inflow Depth = 8.67" for 100-Year event  
 Inflow = 1.75 cfs @ 12.01 hrs, Volume= 5,288 cf  
 Outflow = 0.30 cfs @ 11.62 hrs, Volume= 5,288 cf, Atten= 83%, Lag= 0.0 min  
 Discarded = 0.30 cfs @ 11.62 hrs, Volume= 5,288 cf

Routing by Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs  
 Peak Elev= 2.75' @ 12.42 hrs Surf.Area= 656 sf Storage= 1,145 cf

Plug-Flow detention time= 17.6 min calculated for 5,288 cf (100% of inflow)  
 Center-of-Mass det. time= 17.6 min ( 752.9 - 735.3 )

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	335 cf	<b>20.83'W x 31.50'L x 3.04'H Field A</b> 1,996 cf Overall - 879 cf Embedded = 1,117 cf x 30.0% Voids
#2A	0.50'	879 cf	<b>Cultec R-330XLHD x 16 Inside #1</b> Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows
		1,214 cf	Total Available Storage

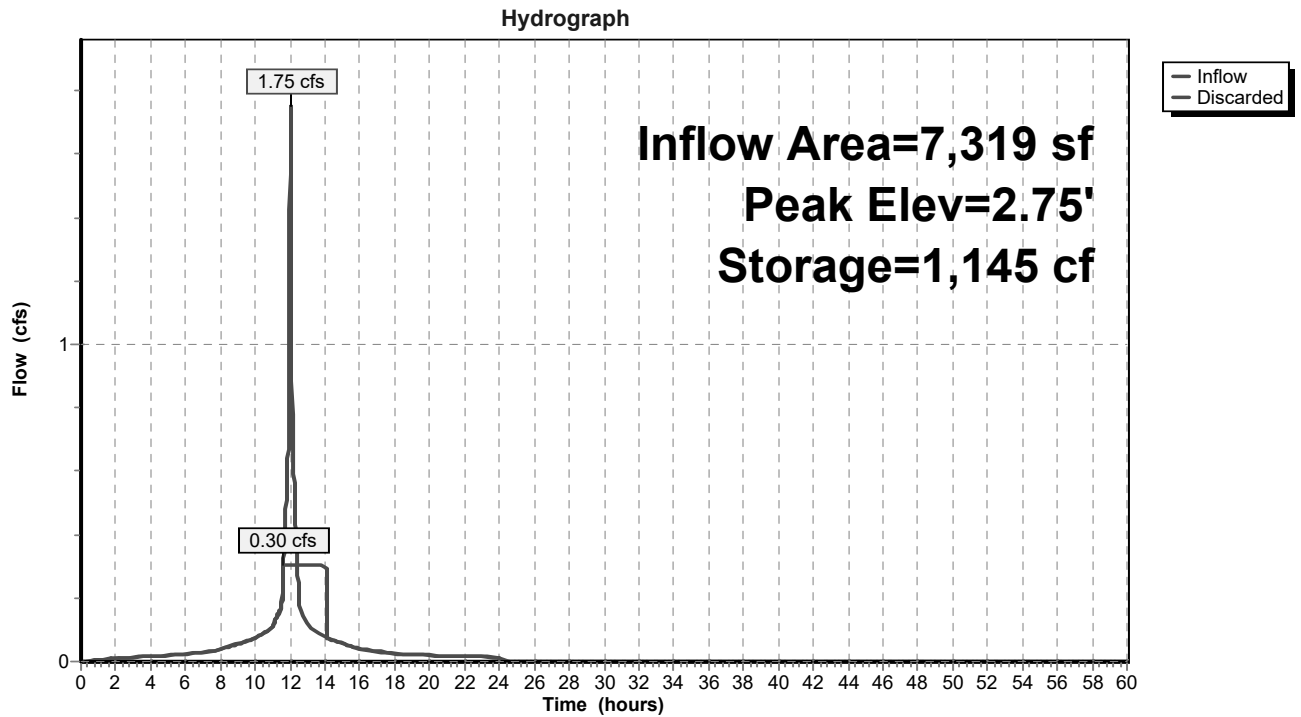
Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	<b>20.000 in/hr Exfiltration over Surface area</b>

Discarded OutFlow Max=0.30 cfs @ 11.62 hrs HW=0.03' (Free Discharge)

↑1=Exfiltration (Exfiltration Controls 0.30 cfs)

**Pond 2P: 16 Cultec R-330XLHD**





**HUDSON**  
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&  
CONSULTING, P.C.

SITE ADDRESS: 34 Clinton Avenue

TOWN/VILLAGE: Dobbs Ferry

DATE: 12/14/2022 TIME: 12:30pm

WEATHER: Sunny TEMP. 36° F

WITNESSED BY: Brendan Williams

**PERCOLATION TEST HOLE DATA SHEET – STORMWATER MANAGEMENT SYSTEM**

Owner

HOLE #	CLOCK TIME				PERCOLATION				
Hole Number	Run No.	Start	Stop	Elapse Time (Min.)	Depth to Water From Ground Surface		Water Level in Inches Drop in inches	Soil Rate	
					Start Inches	Stop Inches		Min. per inch	Inches per Hour
# <u>1</u> 4" Ø	1	12:56	1:07	11	22	46	24	0.46	130.43
	2	1:08	1:24	16	22	46	24	0.67	89.55
	3	1:25	1:42	17	22	46	24	0.71	84.51
	4	1:48	2:07	19	22	46	24	0.79	75.95
	5								
# <u>2</u> 4" Ø	1	12:58	1:26	28	22	46	24	1.17	51.28
	2	1:27	2:03	36	22	46	24	1.5	40
	3	2:06	2:44	38	22	46	24	1.58	37.97
	4	2:46	3:31	45	22	46	24	1.88	31.91
	5	3:32	4:17	45	22	46	24	1.88	31.91
# <u>3</u> 4" Ø	1								
	2								
	3								
	4								
	5								

Notes:

- 1) Tests to be repeated at the same depth until approximately equal soil rates are obtained at each percolation test hole. All data to be submitted for review.
- 2) Depth measurements to be made from top of hole



**HUDSON**  
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&  
CONSULTING, P.C.

SITE ADDRESS: 34 Cliton Avenue

TOWN/VILLAGE: Dobbs Ferry

DATE: 12/13/2022 TIME: 3:45pm

WEATHER: Sunny TEMP. 36° F

WITNESSED BY: Matthew Williams

**DEEP TEST HOLE DATA SHEET – STORMWATER MANAGEMENT SYSTEM**

DEPTH	HOLE NO. <u>1</u>	HOLE NO. <u>2</u>	HOLE NO. <u>3</u>	HOLE NO. <u>4</u>
G.L.	0 – 12"	0 – 12"		
6"	Toposil	Toposil		
12"				
18"				
24"				
30"				
36"	30 – 36"	30 – 36"		
42"	Clay	Clay		
48"				
54"				
60"				
66"				
72"				
78"				
84"				
90"	36 – 96"	36 – 96"		
96"	Sandy loam	Silty loam		
102"	No Ledge	No Ledge		
108"	No GW	No GW		

- Indicate level at which Ground Water (GW), Mottling and/or Ledge Rock is encountered.
- Indicate level for which water level rises after being encountered.

EXCAVATION PERFORMED BY: PRECISION FIELD TESTING